Image: Constraint of the second se

PRELIMINARY GEOTECHNICAL INVESTIGATION

Proposed Subdivision Development

112 - 126 OLD WARRANDYTE ROAD DONVALE, VICTORIA

Prepared for Mullum Pty Ltd C/- Verve Projects Pty Ltd

20 June 2014 G2698.1 AA Ground Science GEOTECHNICAL p +61 3 9464 4617
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ABN 31 105 704 078



PROJECT DETAILS

Project Reference	G2698.1	Rev	AA
Project Title	112 – 126 Old Warrandyte Road		
Project Location	Donvale	State	VIC
Date	20/06/2014		

CLIENT DETAILS

Prepared For (Client)	Verve Projects Pty Ltd		
Project Principal	Mullum Pty Ltd		
Client Address	Level 1, 39 Little Collins Street	Suburb	Melbourne

DISTRIBUTION

Original Held By	Ground Science Pty Ltd
One (1) Electronic Copy	Verve Projects Pty Ltd
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This document presents the results of the preliminary geotechnical investigation conducted for the aforementioned project and is detailed for the sole use of the intended recipient. Should you have any questions related to this report please do not hesitate to contact the undersigned.

AUTHOR:

Gee Singh, Bach. Eng (Hons) Civil

REVIEWED:

Ernie Gmehling, Ass. Dip. (Civil)

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1. INTRODUCTION

Ground Science Pty Ltd (Ground Science) has prepared this report to present the results of the preliminary geotechnical investigation for the proposed subdivision development to be located at 112 – 126 Old Warrandyte Road, Donvale, Victoria (herein referred to as the "site"). The commission was performed at the request of Daniel Dietrich of Verve Projects Pty Ltd (herein referred to as the 'Client') on behalf of Mullum Pty Ltd (herein referred to as the 'Principal').

It is understood that the proposed development will comprise of 56 residential allotments, internal access roads and associated underground services. Minor earthworks to construct building platforms will also form part of the development. The scope of works for this geotechnical investigation was conducted in general accordance with Ground Science proposal GSP2014043 AA dated 15 May 2014, which was prepared in response to the Client's brief dated 9 May 2014.

The professional advice provided in this report is based on the information provided at the time of the report preparation and may not be valid if changes are made to the site, the development proposal or the construction methods. In the event of such changes, further advice should be sought from Ground Science.

2. OBJECTIVES

The objectives of the investigation were as follows:

- Assess the subsurface conditions at the site relevant to the proposed development;
- To provide a preliminary site classification for the proposed allotments;
- Provide preliminary advice regarding suitable founding systems;
- To provide comments and recommendations regarding excavation conditions and construction of controlled fill.

3. SCOPE OF WORKS

3.1 FIELDWORK

The fieldwork was performed on 30 May 2014 and comprised the excavation of 19 test pits (TP1 – TP19) at the locations shown on Figures 1 - 3 in Appendix A. The test pits were excavated with the use of a tracked excavator supplied and operated by Fry's Earthmoving services and were extended to depths of between 0.6m and 2.3m below the existing surface levels. Soil samples were recovered from selected test pits for visual assessment and laboratory testing. On completion the test pits were backfilled with the excavated spoil and surface track rolled with the excavator.

The fieldwork was performed by a suitably qualified engineering geologist from Ground Science who located the test pits, supervised the drilling, recovered test samples and logged the soils.

3.2 LABORATORY TESTING

The laboratory testing comprised of 5 shrink/swell index (I_{SS}) tests, retrieved from the natural soils. The laboratory tests were conducted in Ground Science's NATA accredited in-house testing facility located in Thomastown, Victoria. The results of these tests have been considered in the preparation of this report and



are presented in Appendix C.

4. GEOLOGY

The Geological Survey of Victoria Ringwood mapsheet (scale 1:63,360) indicates the site is situated over Silurian aged Anderson Creek Formation siltstones and sandstones. Quaternary aged alluvial soils are expected to exist adjacent to the Mullum Mullum Creek. It should be noted that this desktop study does not take into account potential fill soils which may exist within the existing earth dams and where existing dwellings are located.

5. SITE DESCRIPTION

The site is bordered by Illawong Drive and Mullum Drive to the south and Old Warrandyte Road to the west and north. Mullum Mullum Creek is located on the northern boundary of the site. Access to the site is provided through Illawong Drive. At the time of our investigation, the topography of the site was generally variable and sloped downward steeply to moderately towards Mullum Mullum Creek. Several trees of different sizes are scattered across the site. Two existing dwellings are located on the eastern and western section of the site. Two existing earth dams are located in the vicinity of the existing dwelling located in the western portion of site and currently retain water. Existing residential dwellings border the site to the east and south. Furrows from obsolete orchards exist on the western and eastern portions of the site. Existing Aboriginal archaeological sites were noted at four different locations. Some field drainage lines were observed during the investigation. At the time of our visit, the surface of the site was trafficable by a 4WD utility vehicle and a tracked excavator.

6. SUBSURFACE PROFILE

The subsurface conditions encountered in the test pits are described in the test pit logs presented in Appendix B. In summary, the soils generally comprised of sandy silts (topsoil) which transitioned into natural silty/sandy/gravelly clays and clayey silts. Extremely weathered sandstone/siltstone deposits generally comprising of silty/sandy clay was observed to underlie the natural residual soils. Penetration refusal was encountered during the performance of most test pits with the exception of test pits TP8 and TP16. The depths at which penetration refusal was encountered is presented in the table below:

Test Pit No	Location	Depth of Penetration Refusal (m)
TP1	Lot 34/35	1.1
TP2	Lot 35/36	1.25
TP3	Lot 34/44	0.6
TP4	Lot 45/46	1.4
TP5	Lot 47/48	0.7
TP6	Lot 48/49/51	1.0
TP7	Lot 54/55	0.8
TP9	Lot 31	1.25

Table 1: Test Pit Penetration Refusal Depth

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Test Pit No	Location	Depth of Penetration Refusal (m)
TP10	Lot 25	1.2
TP11	Lot 27/28	1.0
TP12	Existing Backfilled Swimming Pool	1.3
TP13	Lot 18	0.9
TP14	Lot 22	0.7
TP15	Lot 11/12	0.9
TP17	Lot 2/3	1.35
TP18	Lot 4	1.7
TP19	Lot 12/13	1.3

Groundwater was not encountered in the test pits during the field investigation. It is noted that groundwater levels can vary seasonally and with changes in drainage conditions. Groundwater can also be locally perched within fill materials.

7. RESULTS OF LABORATORY TESTS

The results of the laboratory tests are presented in Appendix C and are summarised in Table 1:

Test Pit	Location	Depth Range (m)	Soil Type	Shrink/Swell Index (%)
TP4	Lot 45/46	0.3 - 0.6	Clayey SILT	0.2
TP8	Lot 37/39/41	0.3 - 0.6	Clayey SILT	0.2
TP9	Lot 31	0.3 - 0.6	CLAY	2.2
TP15	Lot 11/12	0.25 - 0.55	CLAY	1.2
TP18	Lot 4	0.3 - 0.6	Silty CLAY	0.6

Table 2: Laboratory Test Result Summary

8. DISCUSSION & RECOMMENDATIONS

8.1 SUBSURFACE SOIL CONDITIONS - EXISTING BACKFILLED SWIMMING POOL

Test pit TP12 was extended within the existing backfilled swimming pool located adjacent to the existing dwelling on the eastern section of the site. The soils encountered within this test pit generally comprised of uncontrolled fill which consisted of clayey silt, overlying silty clays and silty gravels at deeper depths. Trace fine to coarse sub-angular gravels were observed within the silty clay horizon.

Penetration refusal was encountered at 1.3m below the existing surface level at the time of our investigation which we anticipate to be the concrete base of the existing backfilled swimming pool. It is understood that as part of the construction works, these materials will be removed and the excavation spoils potentially used as



controlled structural fill. Based on our observations within this test pit, the silty clays are considered suitable for use as structural fill provided the soils are assessed as suitable at the time of construction. The clayey silts and silty gravels are not considered suitable for use as structural fill however may be used for nonstructural fill or landscaping purposes. Given the fill soils present in this area, some oversize particles may be present within the soil matrix which should be removed prior to fill placement.

8.2 SUBSURFACE SOIL CONDITIONS - ORCHARD AREA

It is understood that some areas of the site was previously utilised as orchards. Test pits TP1 - TP3 and TP13 were excavated in these areas. The soils encountered within these test pits generally comprised of a thin layer of topsoil (between 0.1m - 0.3m), which overlies natural silty clays. Penetration refusal was encountered in these test pits at depths ranging from 0.6m - 1.25m from existing surface levels.

At the time of our investigation, minor surface roots were observed however noted to not be significant. Excavations in these areas are likely to encounter some surface topsoil and potentially near surface roots before transitioning into the natural soils.

8.3 PRELIMINARY AS2870 SITE CLASSIFICATION

It is understood that the proposed development will comprise of 58 residential allotments. As per discussions with the Client, we understand that these allotments will range from approximately 1,000m² to 3,500m². The proposed building envelope locations are unknown at the time of preparation of this report. Earthworks on this site to form building platforms for the proposed residential dwellings are understood to be minimal. Cut and fill processes are understood to not exceed 1m. It is noted that some trees will be removed and some retained as part of the development.

The subsurface soil profile, level of earthworks, presence of fill, trees and reactivity of the soils will influence the site classification for the proposed allotments.

The test pits have shown a generally natural soil profile comprising a relatively thin topsoil layer, overlying natural silty clays/clayey silts of variable plasticity, which in turn overlie weathered siltstone/sandstone. Penetration refusal was encountered at variable depths over the site as shown in Table 1.

The preliminary site classification was conducted in general accordance with AS2870 (2011) 'Residential Slabs and Footings'. The following site characteristics and parameters were used in the preliminary site classification assessment:

- Climatic Zone (Figure D1): 2;
- Depth of design suction change (H_S): 1.8m;
- Soil Profile Group (Table D1): Group 3.

The site classification is assigned based on an assessment of the characteristic surface movement (y_s) in accordance with the calculation method outlined Section 2.3 of AS2870 (2011). The instability index or shrink swell index (I_{SS}) is the key parameter used in this calculation and is obtained from the laboratory shrink swell index test. For the purpose of this report, we have considered a shrink/swell index of 1.2% and 2.2%.

Based on the laboratory test results, the shrink swell index values were noted to range 0.2% to 2.2% which

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is typical of residual sedimentary deposits. The site classification applicable to various scenarios is further detailed below.

8.3.1 Natural Site

The site classification for a '<u>natural site</u>' applies where a full clay profile depth of 1.8m is achieved (ie no bedrock within 1.8m), no trees are located within the influence distance of the site and the soils are natural (ie no fill).

Based on the results of the geotechnical testing, the geological setting and with reference to Table D1 of AS2870-2011, a site classification of **Class M** is considered applicable with an assessed characteristic surface movement (y_s) in the range of 20mm to 40mm.

8.3.2 Natural Site with Trees

The site classification for a '<u>natural site with trees</u>' applies where a full clay profile depth of 1.8m is achieved (ie no bedrock within 1.8m) and the soils are natural (ie no fill).

A site classification of Class P is considered applicable where an allotment is in close proximity to existing individual or groups of trees which may cause abnormal moisture conditions. In accordance with AS2870 (2011), a site classified as Class P may be reclassified taking into account the presence of trees and reassessing the applicable y_s value.

Based on the results of the geotechnical testing, the geological setting, presence of trees and with reference to Table D1 of AS2870-2011, a site classification of **Class M - Class H1** is considered applicable with an assessed characteristic surface movement (y_s) in the range of 20mm to 40mm (Class M) and 40mm to 60mm (Class H1).

It should be noted that the above site classification is applicable to allotments within the influence distance of trees or in the event that tree(s) have been removed. As a guide, the influence distance should be taken as 1 x the mature height of the tree. This distance should be increased if rows or groups of trees are present.

8.3.3 Controlled Fill Site

The site classification for a '<u>controlled fill site</u>' applies where a site has more than 0.4m of clay fill placed and compacted to Level 1 procedures as detailed in AS3798-2007 'Guidelines on Earthworks for Residential and Commercial Developments'.

For allotments with controlled fill, the site classification will depend on the thickness of the fill and the material used as fill. Where clay or similar cohesive soils are used as controlled fill, the site classification process will need to consider that the cracks that naturally develop within a clay profile over time with seasonal shrinkage and swelling will not be present. The absence of these cracks increases the characteristic surface movement (y_s) over that of a natural clay site. It is recommended that controlled fill materials be carefully selected and reactive clays avoided, where practicable. The onsite clays are considered suitable for use as controlled fill.

For the purpose of this report, we have assumed that the onsite clays as observed in our test pits will be used as controlled fill. Based on the results of the geotechnical testing, the geological setting and with





reference to Table D1 of AS2870-2011, a site classification of **Class M - Class H1** is considered applicable with an assessed characteristic surface movement (y_s) in the range of 20mm to 40mm (Class M) and 40mm to 60mm (Class H1).

8.3.4 Controlled Fill Site with Trees

The site classification for a '<u>controlled fill site with trees</u>' applies where a site has more than 400mm of controlled clay fill and is located within the influence distance of trees or tree groups.

Based on the results of the geotechnical testing, the geological setting, presence of controlled clay fill, presence of trees and with reference to Table D1 of AS2870-2011, a site classification of **Class H1** - **Class H2** is considered applicable with an assessed characteristic surface movement (y_s) in the range of 40mm to 60mm (Class H1) and 60mm to 75mm (Class H2).

Given the potential variable site conditions once the earthworks and removal/retention of trees have been completed, individual allotments will require a site specific site classification based on the actual subsurface profile at the time of construction. Different site classifications may apply for the various allotments across the site.

8.4 BUILDING FOOTINGS

Building footings should be founded beneath any topsoil or uncontrolled fill and within the natural stiff to hard clay soils. Footings founded within these materials may be designed using a maximum allowable bearing pressure of 100kPa. Footings founded on weathered bedrock may be designed using a maximum allowable bearing pressure of 200kPa. The depth, degree and continuity of bedrock should be confirmed by a suitably qualified geotechnical engineer/engineering geologist.

The minimum founding depths for various footing types as presented in AS2870 (2011) should be adopted. Footings should not be founded within any uncontrolled fill.

It should be noted that the use of standard footings in accordance with AS2870 (2011) is only applicable for buildings having loadings and a construction style similar to that of a residential dwelling. Given the reactivity of the subsurface profile, the following precautions should be adopted to assist in the management of reactive soil movements:

- The surface of the site should be graded away from buildings such that run-off drains away and water cannot pond against the building. Where practicable the use paving against the edge of building can reduce the potential for moisture variations;
- Restrict tree planting in the vicinity of the building. On Class M sites trees should be located no closer to the building than 0.75 times their mature height and 1.0 times their mature height on Class H1 and H2 sites. This distance should be increased for groups or rows of trees;
- Plumbing, drainage and other services that have the potential to allow the ingress of water should be avoided beneath buildings. Where service trenches are to pass beneath or near to the building they should be backfilled with a low permeability material, such as compacted clay, to prevent the ingress of water. Any leaking or damaged underground services should be repaired promptly;
- During construction the exposed footing excavations in clay should not be left exposed to the





weather for extended periods. Water should not be allowed to pond in these areas nor should they be left unprotected to dry and crack.

8.5 SUITABLE CONTROLLED FILL MATERIALS

The onsite natural soils are considered suitable for use as controlled fill, subject to an assessment of the soil condition at the time of construction and careful sorting, mixing, as may be required. The on-site soils may be used beneath pavements provided the CBR value is not less than the design value adopted for the pavements, or else the pavements should be redesigned for the actual CBR value of the fill material, and further advice sought.

The onsite clays may be considered for use beneath buildings provided careful consideration is given to the potential for an increase in the reactivity of the profile from its natural condition due to remoulding and the loss of shrinkage cracks, as is discussed in Section 8.1.3 of this report. The low to medium plasticity clays are considered are preferable. Alternative materials may be considered and further advice should be sought from Ground Science where alternative materials are proposed.

All imported soils should have a clean fill certificate. Ground Science can assist with material testing on imported soils.

8.6 BATTER SLOPES

Temporary batter slopes excavated in the natural clays soils up to 3m in height should not be steeper than 1H:1V (45°) and permanent batter slopes of up to 3m in height should not be steeper than 2H:1V (27°). It is noted that flatter batters may be required for maintenance purposes and in areas where uncontrolled fill is present. Further geotechnical advice should be sought should higher or steeper batter slopes be proposed or where batter slopes are required within uncontrolled fill soils.

Batter slopes are likely to be subject to fretting and local loss of material, particularly if exposed to weather for extended periods. Drainage should be provided at the top of batter slopes to divert runoff away from the slope face. Permanent batter slopes should also be protected from erosion by vegetation or proprietary protection systems.

For & on behalf of Ground Science Pty Ltd

Gee Singh BE (Hons) Civil





9. LIMITATIONS

This report only serves as a factual, limited scope geotechnical investigation for the proposed site. It must be noted that due to the limited scope of the investigation, all findings and advice provided with this report are solely for reference purposes. This type of investigation (as per our commission) is not designed or capable of locating all soil conditions, (which can vary even over short distances). The advice given in this report is based on the assumption that the test results are representative of the overall soil conditions. However, it should be noted that actual conditions in some parts of the site might differ from those found. If further sampling reveals soil conditions significantly different from those shown in our findings, Ground Science must be consulted. This report does not constitute a design but may be used by others to include the details for such design.

It is assumed the site is clean of contaminated soils and Ground Science Pty Ltd does not accept any responsibility for any such soils.

It is recognised that the passage of time affects the information and assessment provided in this document. Ground Science's assessment is based on information that existed at the time of the preparation of this document. It is understood that the services provided allowed Ground Science to form no more than an opinion of the actual site conditions observed during sampling and observations of the site visit and cannot be used to assess the effects of any subsequent changes in the quality of the site, or its surroundings, or any laws or regulations.

The scope and the period of Ground Science services are described in the proposal and are subject to restrictions and limitations. Ground Science did not perform a complete assessment of all possible conditions or circumstances that may exist at the Site. If a service is not expressly indicated, it should not be assumed it has been provided. If a matter is not addressed, it should not be assumed that any determination has been made by Ground Science in regards to it.

Where data has been supplied by the client or a third party, it is assumed that the information is correct unless otherwise stated. No responsibility is accepted by Ground Science for incomplete or inaccurate data supplied by others, even if such work has been performed by a third party suggested by Ground Science.

Any drawings or figures presented in this report should be considered only as pictorial evidence of our work. Therefore, unless otherwise stated, any dimensions should not be used for accurate calculations or dimensioning.

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10. REFERENCES

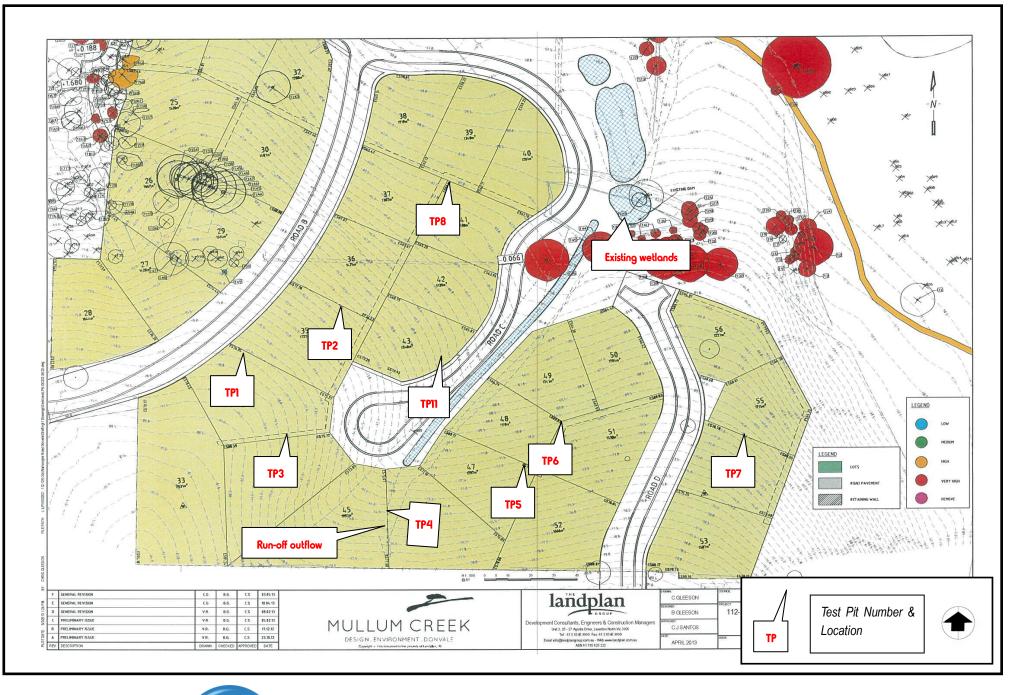
- AS1289 Testing of Soils for Engineering Purposes.
- Geological Survey of Victoria, Ringwood Geological Mapsheet, 1:63,360.
- AS2870 2011 Residential Slabs and Footings.
- AS3798 2007 Guidelines on Earthworks for Residential and Commercial Developments.





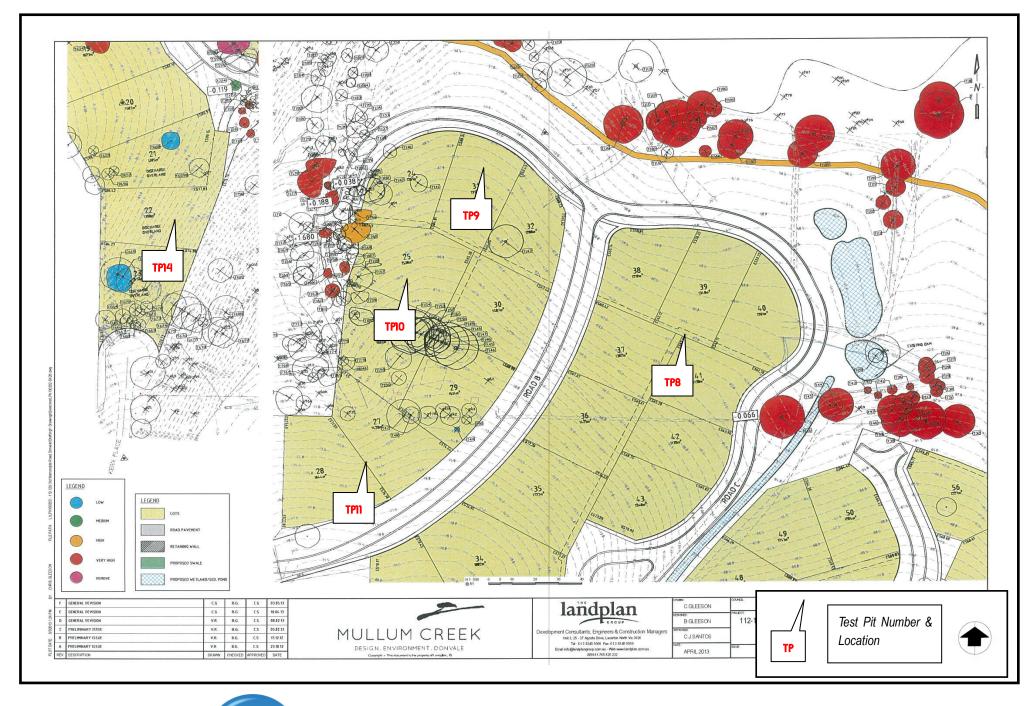
figures

Figure 1: Site Locality Plan

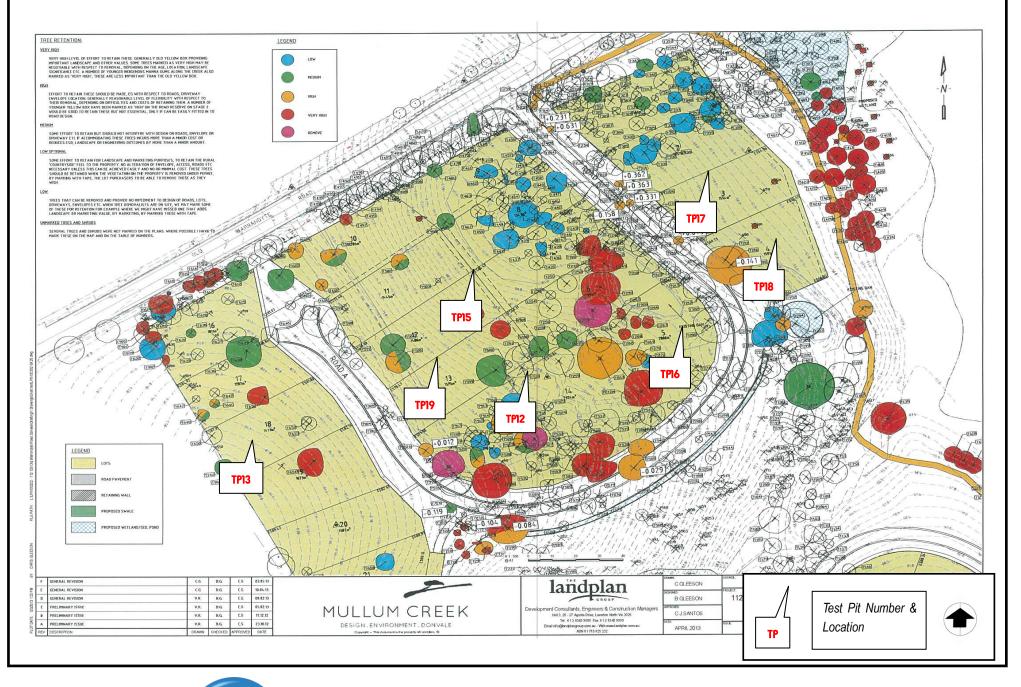


	Project	Details:	,	Drawing No Figure 1	Scale Not to Scale
	112 - 126 OLD WARRANDYTE ROAD	SITE PLAN & TEST LOCATIONS	Drawn		Date
			TH	GS	20 June 2014

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Ground Science	Project		· ·		Scale Not to Scale
	112 - 126 OLD WARRANDYTE ROAD	SITE PLAN & TEST LOCATIONS		Checked	Date
			TH	GS	20 June 2014



	1			Project	Drawing No	Scale
		Project	Details:	G2698.1AA	Figure 3	Not to Scale
nd Science		112 – 126 OLD WARRANDYTE ROAD	SITE PLAN & TEST LOCATIONS	Drawn	Checked	Date
				TH	GS	20 June 2014

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Test Pit Log Sheets

U		Gre			רכפ א נ	Ð	TES	T PIT LOG		Test Pit JOB No		TP1 g2698.1AA	
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DRILL METH	IOD:	Test Pit					EASTING: ND			INCLIN		90°	
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	L	0.90				CL	silty CLAY, low to medium plasticity, light gre	y, with some fine to coars	se gravel	Н	D	Weathered Rock	_
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Q					ICE A L	9	TEST PIT LOG							TP2 g2698.1AA		
CLIENT: PROJECT: LOCATION: TEST LOCA		112 Don	Mullum Pty Ltd c/- Verve Projects Pty Ltd 112 - 126 Old Warrandyte Road Donvale						TEST DATE: 30-May-14 LOGGED BY: TH CHECKED BY: GS							
DRILL MET	HOD:	Te	a 35/36 est Pit					EASTING: ND					INCLINA		90°	
HOLE DIAN		LING		SAMPLING		1		NORTHING: ND		FIELD	MATERIAI	DESCRIPTION	SURFAC	CE RL:	ND	
C PENERTRATION C RESISTANCE	4 WATER	DEPTH (metres)	DEPTH (RL)	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	USC SYMBOL	SOIL /	ROCK MATER	IAL DESCRIPTIO	N		CONSISTENCY DENSITY	MOISTURE	ADDITIONAL OBSERVAT	TIONS
		0.0	0.10					sandy SILT, fine, dark brown silly CLAY, low to medium plasticity, I	light orange, wit	n trace fine to coa	rse grained	subangular grav el	L	Dp : D - Dp	Topsoil Natural Soil	
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																-
		10 - - -	1.00				CL-C	silty CLAY, low to medium plasticity, I	light grey				VSt- H	D	Weathered Rock	
			1.25					Testpit Refusal @ 1.25m								
																- - -
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HAM	LOG	2.0 2009						1	I		DCP	Dynamic Cone F	enerome	eter i est	Sheet 1	1 of 1

ů Č	OUND Scien	ce L	T	EST PIT LOG		Test Pit JOB No		TP3 g2698.1AA	
		Ltd				TEST DA LOGGEL CHECKE	BY:	30-May-14 TH GS	
DRILL METHOD: Test		EASTING:	ND					90°	
HOLE DIAMETER: DRILLING	SAMPLING	NORTHING	: ND	FIELD	MATERIAL DESCRIPTION	SURFAC	E RL:	ND	
2 PENERTRATION 2 RESISTANCE WATER DEPTH (metres) DEPTH (R1)	SAMPLE OR FIELD TEST RECOVERED	GRAPHIC LOG USC SYMBOL	SOIL / I	ROCK MATERIAL DESCRIPTIC	N	CONSISTENCY DENSITY	MOISTURE	ADDITIONAL OBSERVATIO	ONS
		OL sandy SILT,		ght grey and orange, trace line t	o coarse subangular gravel	L	Dp D - Dp	Topsoil Natural Soil	
		CL silty CLAY, 1 Testpit Refus		ght grey and orange, some line	b coarse subangular gravel	VSt- H	D	Weathered Rock	
3.5 PENETRATION	CONSISTENCY	DENSITY		MOISTURE	TEST NOTES				
PENETRATION		v soft Fb VL L v stiff MD	friable very loose loose medium dense dense	MOISTURE D dry Dp damp M moist W wet S saturated	TEST NOTES PP Pocket Penetro U63 Undisturbed Sam D Disturbed Sam Bs Bulk Sample E environmental HSV Hand Shear Vi DCP Dynamic Cone	ample 63mr ple sample ane test	n	on date shown	

	TEST PIT LOG TP4 JOB N0 : G2698.1AA
CLIENT: Mullum Ply Ltd c/- Verve Projects Ply Ltd PROJECT: 112 - 126 Old Warrandy te Road LOCATION: Donvale	TEST DATE: 30-May-14 LOGGED BY: TH CHECKED BY: GS
TEST LOCATION: Lots 45/46 (near water outflow) DRILL METHOD: Test Pit EASTING: N	INCLINATION: 90°
HOLE DIAMETER: NORTHING: N DRILLING SAMPLING	SURFACE RL: ND FIELD MATERIAL DESCRIPTION
PENERTRATION MATER WATER DEPTH (metres) DEPTH (metres) TEST RECOVERED GRAPHIC LOG UUSC SYMBOL	SOIL / ROCK MATERIAL DESCRIPTION
0.0 OL sandy SLT, fine,	
0.10 0.10 10 10 10 10 10 10 10 10 10	medium plasificity, light orange brow n St Dp - M Natural Soil
0.60 CL silty CLAY, low t	edium plasticity, light grey and orange St-VSt Dp
#4 D	
L 1.30 CL silty CLAY, low t	edium plasticity, light grey, with some fine to coarse subangular gravel H D Weathered Rock
_ 1.40 Testpit Refusal @	im line line line line line line line line
15	
2.0	
2.5	
ENETRATION CONSISTENCY DENSITY Vs very soft Fb fria	MOISTURE TEST NOTES D dry PP Pocket Penetrometer Test
1 2 3 4 S soft VL ver St stiff L loo VSt very stiff MD me	bose Dp damp U63 Undisturbed Sample 63mm ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓
no resistingue Frenceset H hard D der	S saturated E environmental sample HSV Hand Shear Vane test DCP Dynamic Cone Penetrometer Test

G							•	Т	EST PI	T LOG			Test Pit JOB No		TP5	
CLIENT: PROJECT: LOCATION		112		.td c/- Verve Proje d Warrandyte Roa		/ Ltd							TEST DA LOGGEI CHECKI	D BY:	30-May-14 TH GS	
TEST LOCA			s 47/48 est Pit					EASTING: ND					INCLINA	ATION:	90°	
HOLE DIAN	DRIL	ING	- 1	SAMPLING		1		NORTHING: ND		FIEL D	MATERIAI	DESCRIPTION	SURFAC	CE RL:	ND	
-					,					FIELD		DESCRIPTION				
 PENERTRATION RESISTANCE 	4 WATER	DEPTH (metres)	DEPTH (RL)	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	USC SYMBOL	SOIL /	ROCK MATERI	AL DESCRIPTIO	N		CONSISTENCY DENSITY	MOISTURE	ADDITIONAL OBSER	VATIONS
		0.0	0.10					sandy SILT, fine, dark brown silty CLAY, medium plasticity, orange	brow n				L St	Dp	Topsoil Natural Soil	
			-	#5	D											-
		0.5														
]	0.60		-		CL	silty CLAY, low to medium plasticity, I	ight grey and or	ange, with some	fine to coars	se subangular gravel	VSt- H	D	Weathered Rock	
	t		0.70			<i>136</i>		Testpit Refusal @ 0.7m								_
		-														
		10														
		_														_
		-														
		15														
		-														
																-
		-														
		2.0														
		-														_
		1														
																_
		2.5														
																_
		-														
		3.0														
		-														
PENETRAT	ION	3.5	1	CONSISTENCY	1			DENSITY	MOISTURE		TEST NOT				<u> </u>	
	1 2	34		Vs S	ver soft	y soft		Fb friable VL very loose	D Dp	dry damp	PP U63	Pocket Penetrom Undisturbed Sam	iple 63m		<u> </u>	•
Ĩ				St VSt	stiff ver	y stiff		L loose MD medium dense	M W	moist wet	D Bs	Disturbed Sample Bulk Sample	9		-	water inflow
l no re	esistence relu			Н	har			D dense	s	saturated	E HSV	environmental sa Hand Shear Van	e test			
HAN	I LOG	2.0 2009							<u> </u>		DCP	Dynamic Cone P	enetrom	eter Tes		et 1 of 1

						9	TEST PIT LOG		Test Pit		TP6
CLIENT: PROJECT: LOCATION:		Mullum Pl 112 - 126 Donvale	y Ltd c/-Verve Proje Old Warrandyte Roa	ects Pt					JOB No TEST DA LOGGED CHECKE	ITE: D BY:	G2698.1AA 30-May-14 TH GS
TEST LOCA		Lots 48/49 Test Pit	9/51				EASTING: ND		INCLINA	TION:	90°
HOLE DIAM	ETER: DRIL		SAMPLING		1		NORTHING: ND	TERIAL DESCRIPTION	SURFAC	E RL:	ND
	DRIL			' T			FIELD MA	TERIAL DESCRIPTION			
C PENERTRATION C RESISTANCE	MATER	DEPTH (metres) DEPTH (RL)	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	USC SYMBOL	SOIL / ROCK MATERIAL DESCRIPTION		CONSISTENCY DENSITY	MOISTURE	ADDITIONAL OBSERVATIONS
		0.0				OL	sandy SLLT, fine, dark brown		L	Dp	Topsoil
		0.10				СІ	silly CLAY, medium plasticity, light grey and orange, with trace fine to med	ium subangular gravel	St		Natural Soil
		0.90	1			CL	silty CLAY, low to medium plasticity, light grey, with some fine to coarse \ensuremath{s}	ubangular grav el	VSt- H	D	Weathered Rock
											-
		2.0									
		-									
		-									
		3.0									
		3.5									
PENETRATI	ON		CONSISTENCY					EST NOTES		•	
no re	1 2	3 4	Vs S St VSt H	sof stiff	f rystif		VL very loose Dp damp L loose M moist I MD medium dense W wet I D dense S saturated I	PP Pocket Penetrom J63 Undisturbed Sample D Disturbed Sample Bulk Sample E environmental sa HSV Hand Shear Van	nple 63mr e mple e test	n	water level veter cutilow veter inflow
HAM	LOG	2.0 2009	1					OCP Dynamic Cone P	enetrome	eter Tes	t Sheet 1 of 1

		Gro	und Sci		е	т	EST PIT LC	ØG	Test Pit JOB No		TP7 G2698.1AA	
LINE DOL 10. Turkin LINE DOL 10. Turking in display Display <thdisplay< th=""> Display Disp</thdisplay<>	LIENT: ROJECT: DCATION:	112 - 126 C Donvale			ł				TEST DA	ITE:) BY:	TH	
DELLIG DAMPING PECLIGUE PECLIGUE DELOGRATION 1990 1	ST LOCATION:					EASTING: ND			INCLINA	TION:	90°	
United billing Unit of the second state of the	DLE DIAMETER:					NORTHING: ND				E RL:	ND	
3 4 8 8 5 4.5 2 5 0 1 <td></td> <td>ING</td> <td></td> <td>,</td> <td></td> <td></td> <td></td> <td>FIELD MATERIAL DESC</td> <td>RIPTION</td> <td></td> <td></td> <td></td>		ING		,				FIELD MATERIAL DESC	RIPTION			
Image: Section of the system of the syste	c PENERTRATION c RESISTANCE WATER	DEPTH (metres) DEPTH (RL)	SAMPLE OR FIELD TEST	RECOVERED GRAPHIC LOG			ROCK MATERIAL DESC	RIPTION	CONSISTENCY		ADDITIONAL OBSER	VATIONS
Image: State in the state is a state in the state in the state is a state in the state in the state is a state in the state in the state is a state in the state in the state is a state in the state in the state is a state in the stat					**		ey and orange					
Under the to codes is defining up or u Image: the top of t			#6	D								-
UND CONSTITUE Description Target Related @ 0.0m Target Related @ 0.0m UND U		-				some fine to coarse subangular grave	I					
Image: Normal State Construct Desiry Molecular 12 1 1 1 12 1 1 1 13 1 1 1 14 1 1 1 15 3.4 1 1 13 1 1 1 14 1 1 1 15 3.4 1 1 15 3.4 1 1 14 1 1 1 15 14 1 1 10 dense 1 1 10 dense 1 1					CL		light grey, with some fine t	to coarse subangular grav	el H	D	Weathered Rock	
Image: Second State State Vis Very State Fb faile Image: Second State State Fb faile Vis Very State Vis Very State Vis Very State Vis Very State Vis		-				Testpit Refusal @ 0.8m						-
Image: Second		-										
Image: Second		_										
Image: Second												
Image: Second		-										
Image: State of the state		15										
Image: State of the state		-										
Image: State of the state		-										
Image: State of the state		-										
1 3.0 -		2.0										
1 3.0 -]										
1 3.0 -		_										
1 3.0 -		-										
1 3.0 -		-										
Image: Stream of the stream		2.5										
Image: Stream of the stream		-										
Image: Stream of the stream		-										
Image: Stream of the stream]										
Image: Stream of the stream		_										
1 2 3 4 S soft VL very loose S soft VL very loose St stiff L loose VSt very stiff MD medium dense H hard D dense S saturated E environmental sample HSV H Hard Hard		3.0										
1 2 3 4 S soft VL very loose S soft VL very loose St stiff L loose VSt very stiff MD medium dense H hard D dense S saturated E environmental sample HSV H Hard Hard		-										
1 2 3 4 S soft VL very loose S soft VL very loose St stiff L loose VSt very stiff MD medium dense H hard D dense S saturated E environmental sample HSV H Hard Hard		-										
1 2 3 4 S soft VL very loose S soft VL very loose St stiff L loose VSt very stiff MD medium dense H hard D dense S saturated E environmental sample HSV H Hard Hard		-										
1 2 3 4 S soft VL very loose Dp damp U63 Undisturbed Sample 63mm Image: St stiff L loose M moist D Disturbed Sample VSt very stiff MD medium dense W wet Bs Bulk Sample water level H hard D dense S saturated E environmental sample HSV Hand Shear Vane test HSV Hand Shear Vane test	NETRATION		1					1	I	1		
St stiff L loose M moist D Disturbed Sample VSt very stiff MD medium dense W Wet Bs Bulk Sample water level on date shown H hard D dense S saturated E environmental sample HSV Hand Shear Vane test HSV Hand Shear Vane test	1 2	3 4			oft						v -4 1	
no resistence H hard D dense S saturated E environmental sample HSV Hand Shear Vane test	٦	.	St	stiff	.ar	L loose	M moist	D Dist	urbed Sample		water level wateroutflow	water inflow
	no resistence refus	al			uñ			led E env	ironmental sample		on date shown	
		I								eter Test		

U	Ground Science GEOTECHNICAL Mullum Ply Ltd c/- Verve Projects Ply Ltd							TEST PIT LOG					Test Pit JOB No		TP8 g2698.1AA			
CLIENT: PROJECT: LOCATION TEST LOCA		112 - Donva	126 Old	d c/- Verve Proje Warrandyte Roa		_td							TEST DA LOGGEL CHECKE	D BY:	30-May-14 TH GS			
DRILL MET	THOD:	Test						EASTING: ND					INCLINA		90°			
HOLE DIA	DRIL	LING	Т	SAMPLING	;			NORTHING: ND		FIELD	MATERIAI	DESCRIPTION	SURFAC	CE RL:	ND			
N					T													
 L PENERTRATION ∞ RESISTANCE 	4 WATER	DEPTH (metres)	DEPTH (KL)	SAMPLE OR FIELD TEST	RECOVERED	_	USC SYMBOL		ROCK MATER	RIAL DESCRIPTION	N		CONSISTENCY DENSITY	MOISTURE	ADDIT	IONAL OBSE	RVATIONS	
		0.0	10		~~~~~	****		sandy SLT, fine, dark brown			14		L	Dp		Topsoil		-
		- 0.	.10			ii ii	L-01	clayey SILT, low to medium plasticity,	moted brown	i, light grey and or	ange, wim	pockets of tine to medi	51- 131	. Dp - Iv		Natural Soil		
			╞	#7	U63	ĺ												_
				#1		ĺ												-
		0.5															ļ	-
			╞		╢													-
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																		_
		3																-
		10															ŀ	
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		-																-
		-																-
		15															Ī	-
																		-
																		_
		- 1.	.80				CI	silty CLAY, medium plasticity, light gre	y, with trace fi	ne sand			VSt	Dp				-
		2.0																-
		_																-
																		-
	+	2	.30		┼∦			Testpit Refusal @ 2.3m						┣				
		1																
		2.5																_
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PENETRAT	10N	3.5	c	ONSISTENCY				DENSITY	MOISTURE		TEST NO	TES	L	L	I			
	1 2	3 4	Τ	Vs S	very soft	soft		Fb friable VL very loose	D Dp	dry damp	PP U63	Pocket Penetrom Undisturbed Sam				-		
				St	stiff			L loose	М	moist	D	Disturbed Sample		•	water level	wateroutflow	water inflow	,
no r	esistence refus			VSt H	very hard	stiff		MD medium dense D dense	W S	wet saturated	Bs E	Bulk Sample environmental sa	mple		water level on date shown	Halel OUIIIOW	water inflow	
											HSV	Hand Shear Van	e test	tor Ta-				
HAN	1106	2.0.2009							1		DCP	Dynamic Cone P	enerome	eter ies	ι	Ch	eet 1 of 1	

	Gro G E	UND Sci		9	т	EST PIT LOG		Test Pit JOB No		TP9 g2698.1AA		
CLIENT: PROJECT: LOCATION: TEST LOCATION:		Ltd c/- Verve Projec Did Warrandyte Road						TEST DA LOGGEI CHECKI	BY:	30-May-14 TH GS		
DRILL METHOD: HOLE DIAMETER:	Test Pit				EASTING: ND NORTHING: ND			INCLINA SURFAC		90° ND		
DRILLING)	SAMPLING				FIELD	MATERIAL DESCRIPTION	SURFAC	ERL:			
L L RESISTANCE F MATER DEPTH (metres)	DEPTH (RL)	SAMPLE OR FIELD TEST	RECOVERED GRAPHIC LOG	USC SYMBOL	Soil /	ROCK MATERIAL DESCRIPTIO	ON	CONSISTENCY DENSITY	MOISTURE	ADDITIONAL OBSERVATIO)NS	
0.0	- 0.10 - - - - - -	#8	U63	8	sandy SiLT, fine, dark brown CLAY, medium plasticity, motifed brow	in, light grey and orange		L St-VSt	Dp	Topsoil Natural Soil		
10	1.10 1.25	#9	D	CL	silly CLAY, low to medium plasticity, I Testpit Refusel @ 1.25m	ight grey, with some fine to coa	rse subangular gravel	VSt- H	D	Weathered Rock		
15	• • • • • • •											
2.5	• • • • • • • • • • • • • • • • • • • •										-	
3.0												
PENETRATION		CONSISTENCY			DENSITY	MOISTURE	TEST NOTES					
HAM LOG 2.02		Vs S St VSt H	very so soft stiff very sti hard		Fb friable VL very loose L loose MD medium dense D dense	D dry Dp damp M moist W wet S saturated	PP Pocket Penetro U63 Undisturbed Sam D Disturbed Sam Bs Bulk Sample E environmental HSV Hand Shear Vi DCP Dynamic Cone	ample 63mr ple sample ane test	n	on date shown	inflow	

Q Q	COUND SCIENCE	TEST PIT LOG	Test Pit No JOB No :	
			TEST DATI LOGGED F CHECKED	
DRILL METHOD: Test	Pit	ASTING: ND	INCLINAT	
HOLE DIAMETER: DRILLING	SAMPLING	IORTHING: ND Field Material Description	SURFACE	ERL: ND
c PENERTRATION c RESISTANCE water Water DEPTH (metres) Depth	DEFITION SAMPLE OR FIELD TEST RECORRED GRAPHIC LOG USS SYMBOL	SOIL / ROCK MATERIAL DESCRIPTION	CONSISTENCY DENSITY	ADDITIONAL OBSERVATIONS
0.0	OL	andy SILT, fine, dark brown illy CLAY, medium plasticity, orange brown	VSt	Dp Topsoil Natural Soil
		illy CLAY, low to medium plasticity, light grey , with some fine to coarse subangular grav el	VSt- H	D Weathered Rock
	CONSISTENCY Vs very soft S soft St stiff VSt very stiff H hard	VL very loose Dp damp U63 Undisturbed L loose M moist D Disturbed St MD medium dense W wet Bs Bulk Sample D dense S saturated E environment HSV Hand Shear HSV Hand Shear HSV Hand Shear	al sample	water level wateroutflow water inflow on date shown

LOCATION: Donvale CHEC TEST LOCATION: Lols 27/28	DATE: ED BY: KED BY: KED BY: ACE RL:	f: GS 1: 90°
DRILL METHOD: Test PR EASTING: ND INCLI HOLE DUMETTER: NORTHING: NO SUPF DRILLING SAMPLING INCLI FIELD MATERIAL DESCRIPTION SUPF UNUSUARS Image: Sampling Image: Sampling <t< th=""><th>ACE RL:</th><th></th></t<>	ACE RL:	
DRILLING SAMPLING FIELD MATERIAL DESCRIPTION I		: ND
NUMULANAL OPENAL OPEN	IOISTURE	
1 2 3 4 2 B 3 C andy SLT, fine, dark brown L 0 0 0.10 0.10 0.10 0.10 C sindy SLT, fine, dark brown and orange SLT. 0.5 0.5 0.5 0.70 0.10 CL-CI sily CLAY, low to medium plasticity, light brown and orange SLT. 0.6 0.70 0.70 0.70 CL-CI sily CLAY, low to medium plasticity, motted grey, brown and orange, with some fine to coarse gravel VSL- 1 1.00 1.00 1.00 Tespil Retural @ 1m 1.00 <	OISTURE	
CL-CL sily CLAY, medium plasticity, light brown and orange St-V	N	ADDITIONAL OBSERVATIONS
CL-Clistity CLAY, low to medium plasticity, motified grey, brown and orange, with some fine to coarse gravel VSt-	Dp St	p Topsoil Natural Soil -
	H D) Weathered Rock
PENETRATION CONSISTENCY DENSITY MOISTURE TEST NOTES VS very soft Fb fiable D dry PP Pocket Penetrometer Te VS very soft S soft VL very loose Dp damp U63 Undisturbed Sample 63 VSt very stiff L loose M moist D Disturbed Sample VSt very stiff MD medium dense W wet Bs Bulk Sample H hard D dense S saturated E environmental sample		vider lovel veter unflow

	U					e	Т	EST PIT	LOG			Test Pit I JOB No		TP12 G2698.1AA	
Disk Letting: Turk /T Mathem in 0 Machanom	PROJECT: LOCATION:	112 Dor	2 - 126 Olo nvale	d Warrandy te Roa		d						LOGGED	BY:	тн	
IMPLAND IMPLAND INCOMENDATION INCOMENDATION 1	DRILL METHOD	D: T													
1 2 3 4 2 8 0 3. 2 2 8 5 0 1.5 3 8 5 1 2 3 4 2 6 - <t< td=""><td></td><td></td><td></td><td>SAMPLING</td><td>i</td><td></td><td>NORTHING: ND</td><td></td><td>FIELD I</td><td>MATERIAL</td><td>DESCRIPTION</td><td>SURFAC</td><td>E RL:</td><td>ND</td><td></td></t<>				SAMPLING	i		NORTHING: ND		FIELD I	MATERIAL	DESCRIPTION	SURFAC	E RL:	ND	
1 2 3 4 2 8 0 3. 2 2 8 5 0 1.5 3 8 5 1 2 3 4 2 6 - <t< td=""><td>NC</td><td></td><td></td><td>0</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>	NC			0											
	L PENERTRATI	WATER DEPTH (metres)	DEPTH (RL)	Sample or Fieli Test	RECOVERED					N					IONS
Image: Second State Construction Construction Provide Provide State Construction MO D Image: Second State Construction Image: Second State Construction Image: Second State Construction MO D Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image: Second State Construction Image:			0.30	#10						ne to coarse	subangular grav el			Uncontrolled Fill	-
PEREMANDA CONSISTENCY DEMITY MOSUBE TEST NOTES PEREMANDA CONSISTENCY DEMITY MOSUBE TEST NOTES V1 2.3 S St St St St St St T V VY		0.5	-												
PEREMANDA CONSISTENCY DEMITY MOSUBE TEST NOTES PEREMANDA CONSISTENCY DEMITY MOSUBE TEST NOTES V1 2.3 S St St St St St St T V VY															-
PENETRATION CONSISTENCY DENSITY MOSTURE TEST NOTES		-	1.10			GM	silty GRAVEL, fine to coarse, brown, ș	grey and orange				MD	D	-	_
PENETRATION CONSISTENCY DENSITY MOISTURE TEST NOTES PENETRATION CONSISTENCY DENSITY MOISTURE TEST NOTES Vs very soft Fb fiable D dry PP Pocket Peneformet Test Vs very soft Fb fiable Dp dry US3 Undistribed Sample Indistribed Sample VS1 very siff MD medium dense W wet Bs Buk Sample medium dense wet			1.30				Testpit Refusal @ 1.3m								
PENETRATION CONSISTENCY DENSITY MOISTURE TEST NOTES PENETRATION CONSISTENCY DENSITY MOISTURE TEST NOTES Vs very soft Fb fiable D dry PP Pocket Peneformet Test Vs very soft Fb fiable Dp dry US3 Undistribed Sample Indistribed Sample VS1 very siff MD medium dense W wet Bs Buk Sample medium dense wet															-
PENETRATION CONSISTENCY Density MOISTURE TEST NOTES PENETRATION CONSISTENCY Density MOISTURE TEST NOTES VS very soft Fb fiable D drap PP VS very loose D drap UG3 Undisturbed Sample Test mortes VSt very stiff MD medium dense W wet Bs Buk Sample weter find weter under wete		2.0													
PENETRATION CONSISTENCY Density MOISTURE TEST NOTES PENETRATION CONSISTENCY Density MOISTURE TEST NOTES VS very soft Fb fiable D drap PP VS very loose D drap UG3 Undisturbed Sample Test mortes VSt very stiff MD medium dense W wet Bs Buk Sample weter find weter under wete															-
PENETRATION CONSISTENCY DENSITY MOISTURE TEST NOTES PENETRATION CONSISTENCY DENSITY MOISTURE TEST NOTES VS very soft Fb fiable D dry PP Pocket Penetrometer Test VS very soft Fb fiable Dp damp UG3 Undisturbed Sample 63mm Image: Construction of the extended		2.5													-
PENETRATION CONSISTENCY DENSITY MOISTURE TEST NOTES PENETRATION CONSISTENCY DENSITY MOISTURE TEST NOTES VS very soft Fb friable D dry PP POLICITATION VS very soft V Very loose D dampe 63mm S soft L loose M moist D Disturbed Sample 63mm VSt very stiff MD medium dense W Wet Bs Bulk Sample															-
Vs very soft Fb friable D dry PP Pocket Penetrometer Test 1 2 3 4 S soft VL very loose Dp damp U63 Undisturbed Sample 63mm ▲ ▲ ▲ ▲ St stiff L loose M moist D Disturbed Sample water load		3.0													
Vs very soft Fb friable D dry PP Pocket Penetrometer Test 1 2 3 4 S soft VL very loose Dp damp U63 Undisturbed Sample 63mm ▲ ▲ ▲ St stiff L loose M moist D Disturbed Sample water load water<															
1 2 3 4 Vs very soft Fb friable D dry PP Pocket Penetrometer Test S soft VL very loose St stiff L loose VSt very stiff MD medium dense W wet Bs Bulk Sample		3.5													
HSV Hand Shear Vane test	ı الح	٦.	(Vs S St VSt	soft stiff very s		Fb friable VL very loose L loose MD medium dense	D Dp M r W	damp noist wet	PP U63 D Bs E	Pocket Penetrom Undisturbed Sam Disturbed Sample Bulk Sample environmental sa	nple 63mr e ample		water level watercution weter	er inflow

U				und Sci		1 CE	•	Т	EST PI	TLOG			Test Pit JOB No		TP13 G2698.1AA	
CLIENT: PROJECT: LOCATION: TEST LOCA		11: Do		Ltd c/- Verv e Proje Id Warrandy te Road		y Ltd							TEST DA LOGGEI CHECKI	D BY:	30-May-14 TH GS	
DRILL MET			lest Pit					EASTING: ND					INCLIN	ATION:	90°	
HOLE DIAM	DRIL			SAMPLING	;	T		NORTHING: ND		FIELD	MATERIAL	DESCRIPTION	SURFAC	CE RL:	ND	
N																
 PENERTRATION RESISTANCE 	A	DEPTH (metres)	DEPTH (RL)	Sample or Field Test	RECOVERED	GRAPHIC LOG	USC SYMBOL		ROCK MATER	IAL DESCRIPTIO	N		CONSISTENCY DENSITY	MOISTURE	ADDITIONAL OBSERV	ATIONS
		0.0	0.10					sandy SILT, fine, dark brown					L	Dp	Topsoil	
			0.10				CL	gravelly CLAY, low to medium plastic	ty, light grey m	ottled yellow, gra	vel fine to c	parse grained	VSt- H	D	Natural Soil	-
		0.5		#11	D											
			0.80					silty CLAY, low plasticity, light grey m	ottled orange, w	vith some fine to c	oarse suba	ngular gravel	н		Weathered Rock	
	-	-	0.90		_			Testpit Refusal @ 0.9m				.3 3		-		
		10														
																_
		15														
																-
		2.0														_
																-
		-														
		2.5														-
																-
		3.0														
		-														
PENETRATI	ON	3.5		CONSISTENCY				DENSITY	MOISTURE		TEST NO	TES				
- 2012 1100 11		3 4		Vs		ry sof	t	Fb friable	D	dry	PP	Pocket Penetror				
₽				S St	sof stiff			VL very loose L loose	Dp M	damp moist	U63 D	Undisturbed Sam Disturbed Samp		11	<u>⊥</u>	•
no re	sistence	a l		VSt H	ver har	ry stiff rd		MD medium dense D dense	W S	wet saturated	Bs E	Bulk Sample environmental s			water level wateroutflow v on date shown	vater inflow
		I									HSV DCP	Hand Shear Va Dynamic Cone		eter Tes		
HAM	_LOG_	2.0 2009													Shee	t1 of 1

G		Gro			1CE A L	9	TE	ST PIT LO	G		Test Pit JOB No		TP14 G2698.1AA	
CLIENT: PROJECT: LOCATION:		112 - 126 (Donvale	r Ltd c/- Verve Proj Old Warrandyte Roa		ty Ltd						TEST DA LOGGEI CHECK		30-May-14 TH GS	
TEST LOCATIO		Lot 22 Test Pit					EASTING: ND				INCLIN	ATION:	90°	
HOLE DIAME	TER:	ING	SAMPLIN	G	Т		NORTHING: ND	F		AL DESCRIPTION	SURFA	CE RL:	ND	
 C PENERTRATION C RESISTANCE F 	WATER	DEPTH (metres) DEPTH (RL)	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	USC SYMBOL	SOIL / RC	OCK MATERIAL DESCR	IPTION		CONSISTENCY DENSITY	MOISTURE	ADDITIONAL OBSERVATIONS	
		D.0				OL	sandy SILT, fine, dark brown, fibrous org	anic inclusions (root 100	mm dia) within	top 200mm	L	Dp	Topsoil	
		0.20				CL	silty CLAY, low to medium plasticity, ligh	t grey mottled orange an	d brown, some	e fine to medium gravel	VSt	D - Dp	Natural Soll	
		0.60					silty CLAY, low to medium plasticity, light	t grey mottled orange, w	ith some fine to	o coarse gravel	Н	D	Weathered Rock	
		0.70					Testpit Refusal @ 0.7m] -
		-												
		10												
		_												
		_												
		_												
		-												
		15												-
		-												
		-												-
		-												
		-												
		2.0												
		-												
		_												-
		-												
		2.5												-
		-												
		_												_
		-												
		-												
		3.0												
		-												
		_												
		-												
		_												
PENETRATIO	N	3.5	CONSISTENCY		<u> </u>	<u> </u>	DENSITY	OISTURE	TEST N	IOTES	<u> </u>	I	l	<u> </u>
	12	3 4	Vs S	ver sof	ry so' A	ft	Fb friable VL very loose	D dry Dp damp	PP U63	Pocket Penetror Undisturbed Sa				
۔ اح	5		St	stiff	f	_	L loose	M moist	D	Disturbed Samp				
no resis	stence refusa	•	VSt H	ver har	ry stif rd	f	MD medium dense D dense	W wet S saturate		Bulk Sample environmental s			water level wateroutflow water inflo on date shown	w
		I							HSV DCP	Hand Shear Va Dynamic Cone		eter Test		
HAM_L	_OG_2	.0 2009					•		50.	,			Sheet 1 of 1	

			Scie H N I C	nce A L	e	Т	EST PIT L	OG		Test Pit JOB No		TP15 G2698.1AA		
					Pty Ltd	I						ATE: D BY: ED BY:	30-May-14 TH GS	
DRILL MET	HOD:	Test					EASTING: ND				INCLIN		90°	
HOLE DIAN		LING	SA	MPLING			NORTHING: ND		FIELD N	IATERIAL DESCRIP	SURFA	E RL:	ND	
PENERTRATION RESISTANCE	4 WATER	DEPTH (metres)	UEPTH (KL) SAMPLE OR FIELD	TEST	RECOVERED GRAPHIC LOG	USC SYMBOL	SOIL /	ROCK MATERIAL D	ESCRIPTION		CONSISTENCY DENSITY	MOISTURE	ADDITIONAL OBSERVATIO	ONS
123	4 🕅	0.0	<u>8</u>		2 0		sandy SILT, fine, dark brown					⊃ Dp	Topsoil	T
		-	25 #	12 U	63	CL-C	ICLAY, low to medium plasticity, orang	ge brown			VSt		Natural Soil	
		0.5 - - -	60			CL	silty CLAY, low to medium plasticity, o	orange brow n, with se	ome fine to co	oarse subangular grav	rel H	D	Weathered Rock	
	Ī	10 10	90				Testpit Refusal @ 0.9m							_
														-
		15												
														-
		2.5												-
														-
		- 3.0 - -												
PENETRATI		3.5	CONSIST	ENCY			DENSITY	MOISTURE		TEST NOTES				
^		3 4	Vs S St VSt H	v si si v	ery so oft tiff ery sti ard		Fb friable VL very loose L loose MD medium dense D dense	D dr Dp da M mo W we	mp ist	PP Pocket U63 Undistu D Disturbe Bs Bulk Sa E environ HSV Hand S	mental sample hear Vane test	n	on date shown	inflow
HAM	LOG	2.0 2009					1	1		DCP Dynami	c Cone Penetrom	eter l'es	t Sheet 1 c	of 1

)CE	9	;	TEST PIT LOG						TP16			
PROJECT: 112 - 126 O LOCATION: Donvale			Ltd c/- Verve Proje Id Warrandy te Road		y Ltd							JOB No TEST DA LOGGEL CHECKE	ATE: D BY:	30-May -14 TH		
TEST LOCATION: Lot 5, near dam DRILL METHOD: Test Pit EASTING								EASTING: ND					INCLINA	ATION:	90°	
HOLE DIA!		t: LLING		SAMPLING	;	Γ		NORTHING: ND		FIELD	MATERIAL	DESCRIPTION	SURFAC	CE RL:	ND	
z				_												
 PENERTRATION RESISTANCE 	4 WATER	DEPTH (metres)	DEPTH (RL)	Sample or Field Test	RECOVERED	GRAPHIC LOG	USC SYMBOL	SOIL	/ ROCK MATERI	AL DESCRIPTIO	N		CONSISTENCY DENSITY	MOISTURE	ADDITIONAL OBS	ERVATIONS
		0.0					OL	sandy SILT, fine, dark brown					L	Dp	Topsoil	
		-	0.20				CI-CF	silty CLAY, medium to high plasticity	, orange brown m	iotted red			St		Natural Soil	
		-		#13	D											
		-														
		0.5	1													
		_	1													
		-	-													
		-														
		10														
		-	1.20				CI	silty CLAY, low to medium plasticity	light grey and or	ance, with some	fine sand		VSt			
							-									-
		-														
		15														
		-														
		-														-
			1.90				CL	sandy CLAY, low to medium plastici	ty, light grey				н	D	-	
		2.0	2.00			7%		Testpit Terminated @ 2m								
		-														
		-														-
		-														
		2.5														-
		-														
		_														-
		-														
		3.0														
		-														
		-														
		-	-													
ENETRAT	ION	3.3		CONSISTENCY	-			DENSITY	MOISTURE		TEST NO			1	1	I
د	1 2	34		Vs S	soft		τ	Fb friable VL very loose	D Dp	dry damp	PP U63	Pocket Penetrom Undisturbed San	nple 63mr		⊥ ◄	
		٦.	٩	St VSt		ry stif	F	L loose MD medium dense	M W	moist wet	D Bs	Disturbed Sample Bulk Sample			water level wateroutflow on date shown	water inflow
no r	esistenç refi	e usal		н	har	rd		D dense	S	saturated	E HSV	environmental sa Hand Shear Van				
	1100	2 0 200									DCP	Dynamic Cone P		eter Tes		heat 1 of 1

		TEST PIT LOG	Test Pit I	No	TP17			
CLIENT: Mullum Pt	O T E C H N I C A L y Ltd c/- Verve Projects Pty Ltd Old Warrandyte Road		JOB No : G2698.1AA TEST DATE: 30-May-14 LOGGED BY: TH CHECKED BY: GS					
TEST LOCATION: Lots 2/3 DRILL METHOD: Test Pit		EASTING: ND	INCLINA	TION:	90°			
HOLE DIAMETER:	I	NORTHING: ND	SURFAC		ND			
DRILLING	SAMPLING	FIELD MATERIAL DESCRIPT						
C PENERTRATION C RESISTANCE MATER DEPTH (metres) DEPTH (RL)	SAMPLE OR FIELD TEST RECOVERED GRAPHIC LOG ULSC SYMBOL	SOIL / ROCK MATERIAL DESCRIPTION	CONSISTENCY	MOISTURE	ADDITIONAL OBSERVATIONS			
0.0		sandy SLT, fine, dark brown	L	Dp	Topsoil			
0.20		H CLAY, medium to high plasticity, motified red, brown, grey and orange	St	Dp - M	Netural Soil			
1.10	#14 D Cl	silly CLAY, low to medium plasticity, light grey, with some fine to coarse subangular gravel	Н	D				
		Testpi Refusal @ 1.35m						
PENETRATION	CONSISTENCY Vs very soft S soft St stiff VSt very stiff H hard	VL very loose Dp damp U63 Undistur L loose M moist D Disturbe MD medium dense W wet Bs Bulk Sar D dense S saturated E environr HAND Hand Sh Hand Sh Hand Sh Hand Sh	Penetrometer Test bed Sample 63mr d Sample nple nental sample near Vane test cone Penetrome	n	vester level vester cuttow vester intow on date shown			

		dSci	TEST PIT LOG								Test Pit JOB No		TP18 G2698.1AA			
CLIENT: Mullum Pty L PROJECT: 112 - 126 Old LOCATION: Donvale TEST LOCATION: Loc4			126 Old War			Ltd									30-May-14 TH GS	
	EST LOCATION: Lot 4 ORILL METHOD: Test Pit						EASTING: ND					INCLIN	ATION:	90°		
HOLE DIAM	ETER: DRILI	ING		SAMPLING	;	1		NORTHING: ND		FIELD	MATERIAL	DESCRIPTION	SURFA	CE RL:	ND	
 PENERTRATION RESISTANCE 	WATER	DEPTH (metres)	иертн (кс)	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	USC SYMBOL		SOIL / ROCK MATER	RIAL DESCRIPTIO	N		CONSISTENCY DENSITY	MOISTURE	ADDITIONAL OBSER	VATIONS
		0.0 - -					OL	sandy SILT, fine, dark brown					L	Dp	Topsoil	
		0	.20	#15	U63		CL	sandy CLAY, low to medium	plasticity, light grey, sa	nd is fine grained			F - St	Dp - M	Natural Soil	-
		0.5	.60				CI	silty CLAY, medium to high p	lesticity mottled brown	arey and orange			St	Dp		
							01	any obst, moduli to ingi p	asieny, moued brown,	grey and orange				55		-
		10														_
																-
		-														
I.		15	.60				CL	silty CLAY, low to medium pl	asticity, light grey, som	e fine to coarse su	bangular gr	avel	Н	D	Weathered Rock	
		- 1	.70					Testpit Refusal @ 1.7m								-
		2.0														_
																-
		-														
		2.5														
																-
		3.0														
		-														
ENETRATIO	ON	3.5	CON	SISTENCY				DENSITY	MOISTURE		TEST NO	TES		I	l	I
\mathbf{r}	1 2	l	V: S S: V: H	t St	soft stiff	y stiff		Fb friable VL very loose L loose MD medium dense D dense	D Dp M W S	dry damp moist wet saturated	PP U63 D Bs E HSV	Pocket Penetrom Undisturbed Samp Disturbed Sampl Bulk Sample environmental sa Hand Shear Var	nple 63m e ample		water level wateroutflow	water inflow
Нам	106.1	2.0 2009									DCP	Dynamic Cone F		eter Test		et 1 of 1

			rou	UND Science TEST PIT LOG											TP19			
Y			GEO	OTECHNICAL											G2698.1AA			
CLIENT: Multurn Pty Ltd c/- Verve Projects Pty Ltd PROJECT: 112 - 126 Old Warrandyte Road LOCATION: Donvale													TEST DA LOGGED CHECKE	BY:	30-May-14 TH GS			
EST LOCA		Lots Tes						EASTING: ND						TION	90°			
IOLE DIAM		Tes	it Pit					NORTHING: ND					INCLINA SURFAC		90 ND			
	DRIL	ING		SAMPLING	;		1	1		FIELD	MATERIAI	DESCRIPTION	1	r –				
PENERTRATION RESISTANCE	ſ	DEPTH (metres)	H (RL)	Sample or Field Test	RECOVERED	GRAPHIC LOG	SYMBOL	SOIL /	ROCK MATER	IAL DESCRIPTIC	N		CONSISTENCY DENSITY	URE	ADDITIONAL OBSERVATION	NS		
1234	WATER	DEPTH	DЕРТН (RL)	SAMPI TEST	RECO'	GRAPI	USC S						CONS	MOISTURE				
		0.0 -					OL	sandy SILT, fine, dark brown					L	Dp	Topsoil			
		0	.20				CI	silty CLAY, low to medium plasticity, I	ight grey and or	range brow n, sor	ne fine to co	arse gravel	St - VSt		Natural Soil	-		
													10					
		0.5	.40					silty CLAY, medium plasticity, orange	brow n				VSt					
		-																
		-																
		-																
		-																
		10 1	.00					some fine to coarse subangular gravel					-			-		
		-																
		1	.20				CL	sandy CLAY, low to medium plasticity	, light grey , sor	me fine to coarse	subangular	grav el	Н	D	Weathered Rock	_		
	t	_ 1	.30			772		Testpit Refusal @ 1.3m								_		
		-																
		15																
		-																
		-																
		-																
		2.0																
		-																
		_																
		2.5																
		-																
		_																
		-																
		3.0																
]																
		_																
		-																
		3.5									1							
ENETRATIO	N		c	Vs	ver	ry sof	t	DENSITY Fb friable	MOISTURE D	dry	TEST NO PP	TES Pocket Penetron	neter Test					
÷	1 2	3 4		S	sof	t		VL very loose	Dp	damp	U63	Undisturbed Sar	nple 63mr		⊻ ◄ ►			
ſ				St VSt	stiff ver	ry stiff		L loose MD medium dense	M W	moist wet	D Bs	Disturbed Samp Bulk Sample	le		water level wateroutflow water in on date shown	inflow		
l no res	sistence refus	a		Н	har			D dense	S	saturated	Е	environmental sa						
							I				HSV DCP	Hand Shear Var Dynamic Cone F		eter Tes	st			
нам	1.06	2.0 2009						-							Sheet 1 of	f 1		



Laboratory Results



A C N 105 704 078 13 Brock Street Thomastown VIC, P 03 9464 4617 F 03 9464 4618

Client:	MULLUM PTY LTD C/- VERVE PROJECTS PTY LTD	job No.	G2698.1
project:	112 - 126 WARRANDYTE RD	report No.	AA
location:	DONVALE	test date:	3-Jun-14

SHRINK / SWELL TEST RESULTS

sample identification :

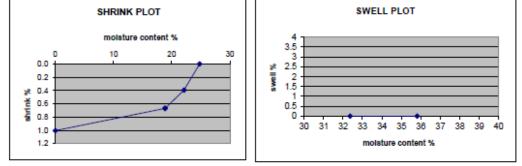
3 (LOTS 45 / 46) depth (m) TP4 @ 0.3 - 0.6

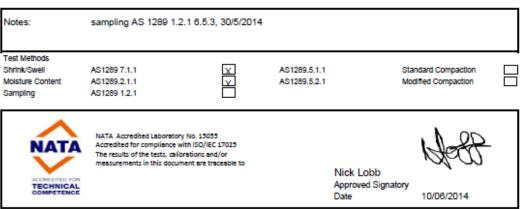
sample description: silty CLAY, medium plasticity, grey, trace well weathered gravel

SHRINK TEST

SWELL TEST

bulk density of core s	pecimen		moisture co	ontent (%)	estimated UCS	strength (kPa)
1.954	t/m ³		before test	after test	before test	after test
moisture content%	32.4		32.4	35.8	75	90
shrink on		estimated inert			swell on	shrink / swell
drying (%)		material (%)			saturation(%)	index: I _{ss} (%)
1.0		10.0			0.0	0.6
amount of crumbling		amount of cracking				
during shrinkage		during shrinkage				
very slight		very slight				







A C N 105 704 078 own VIC P 03 9464 4617 F 03 9464 4618 12 Decels Stee

1	TO DRUCK SUBELTIN	UTIASIUNT VIC, E US 3404 4017 E US 3404 4010	
	Client	MULTUM PTY I TD C/- VERVE PROJECTS PTY I	TD

Client:	MULLUM PTY LTD C/- VERVE PROJECTS PTY LTD	job No.	G2698.1
project:	112 - 126 WARRANDYTE RD	report No.	AB
location:	DONVALE	test date:	3-Jun-14

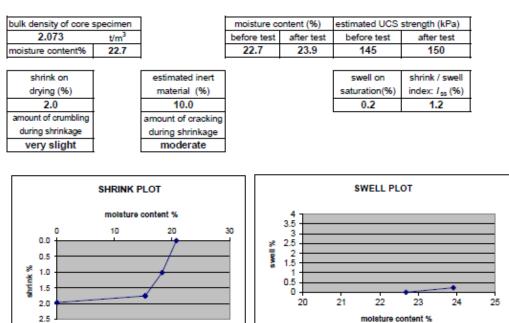
SHRINK / SWELL TEST RESULTS

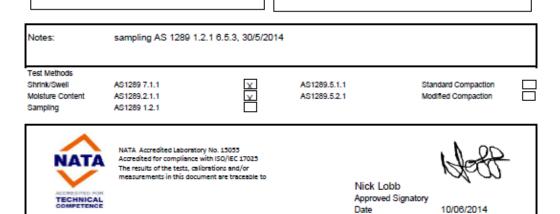
sample identification : sample description:

SHRINK TEST

7 (LOTS 37, 39, 4 depth (m) TP8 @ 0.3 - 0.6 CLAY, high plasticity, mottled red orange brown

SWELL TEST





Date



A C N 105 704 078 13 Brock Street Thomastown VIC, P 03 9464 4617 F 03 9464 4618

Client:	MULLUM PTY LTD C/- VERVE PROJECTS PTY LTD	job No.	G2698.1	
project:	112 - 126 WARRANDYTE RD	report No.	AC	
location:	DONVALE	test date:	3-Jun-14	
	SHRINK / SWELL TEST RESULTS			

HRINK 7 SWELL TEST RESULTS

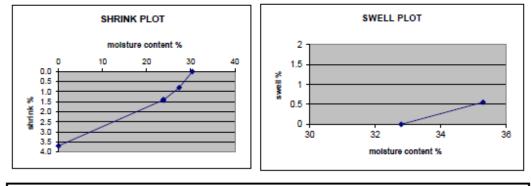
sample identification : sample description: #8 (LOTS 31) depth (m) TP9 @ 0.3 - 0.6

ion: CLAY, high plasticity, mottled red orange brown

SHRINK TEST

SWELL TEST

bulk density of core s	pecimen		moisture co	ontent (%)	estimated UCS	strength (kPa)
1.902	t/m ³		before test	after test	before test	after test
moisture content%	32.8]	32.8	35.3	260	190
i						
shrink on		estimated inert			swell on	shrink / swell
drying (%)		material (%)			saturation(%)	index: I _{ss} (%)
3.7		0.0			0.6	2.2
amount of crumbling		amount of cracking	1			
during shrinkage		during shrinkage				
none		high				



Notes:	sampling AS 1289 1.2.1 6.	5.3, 30/5/2014	ł		
Test Methods Shrink/Swell Moisture Content Sampling	AS1289 7.1.1 AS1289.2.1.1 AS1289 1.2.1	X	AS1289.5.1.1 AS1289.5.2.1	Standard Compaction Modified Compaction	
	NATA Accredited Laboratory No. 1 Accredited for compliance with ISC The results of the tests, calibration measurements in this document ar)/IEC 17025 s and/or	Nick Appro Date	Lobb ved Signatory 10/06/2014	



A C N 105 704 078 13 Brock Street Thomastown VIC, P 03 9464 4617 F 03 9464 4618

Client:	MULLUM PTY LTD C/- VERVE PROJECTS PTY LTD	job No.	G2698.1
project:	112 - 126 WARRANDYTE RD	report No.	AD
location:	DONVALE	test date:	3-Jun-14

SHRINK / SWELL TEST RESULTS

sample identification :

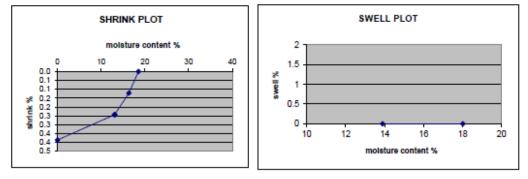
12 (LOTS 11, 12) depth (m) TP15 @ 0.25- 0.55

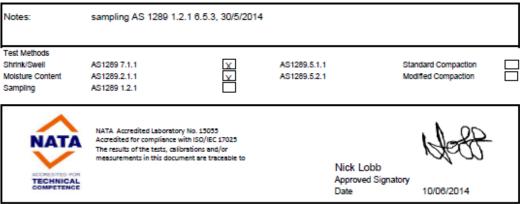
sample description: clayey SILT, medium plasticity, orange/brown, with weathered gravel

SHRINK TEST

SWELL TEST

bulk density of core s	pecimen		moisture co	ontent (%)	estimated UCS	strength (kPa)
1.903	t/m ³]	before test	after test	before test	after test
moisture content%	13.9]	13.9	18.0	275	300
shrink on		estimated inert			swell on	shrink / swell
drying (%)		material (%)			saturation(%)	index: I _{ss} (%)
0.4		15.0			0.0	0.2
amount of crumbling		amount of cracking				
during shrinkage		during shrinkage				
moderate		slight				





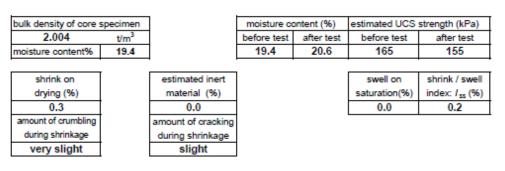


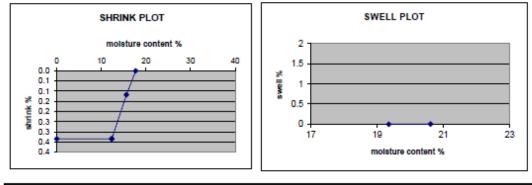
A C N 105 704 078 13 Brock Street Thomastown VIC, P 03 9464 4617 F 03 9464 4618

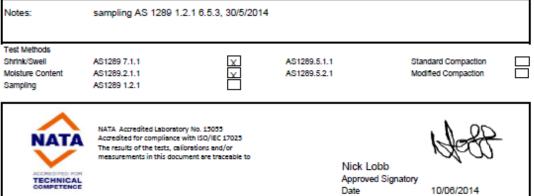
project: 112 - 126 WARRANDYTE RD location: DONVALE SHRINK / SWELL TEST RESULTS sample identification : #14 (LOT 4) depth (m)	brown, trace gravel		
location: DONVALE	TP18 @ 0.3 - 0.6		
project: 112 - 126 WARRANDYTE RD	test date:	3-Jun-14	
	report No.	AE	
Client: MULLUM PTY LTD C/- VERVE PROJECT	PTY LTD job No.	G2698.1	

SHRINK TEST

SWELL TEST









Site Photographs



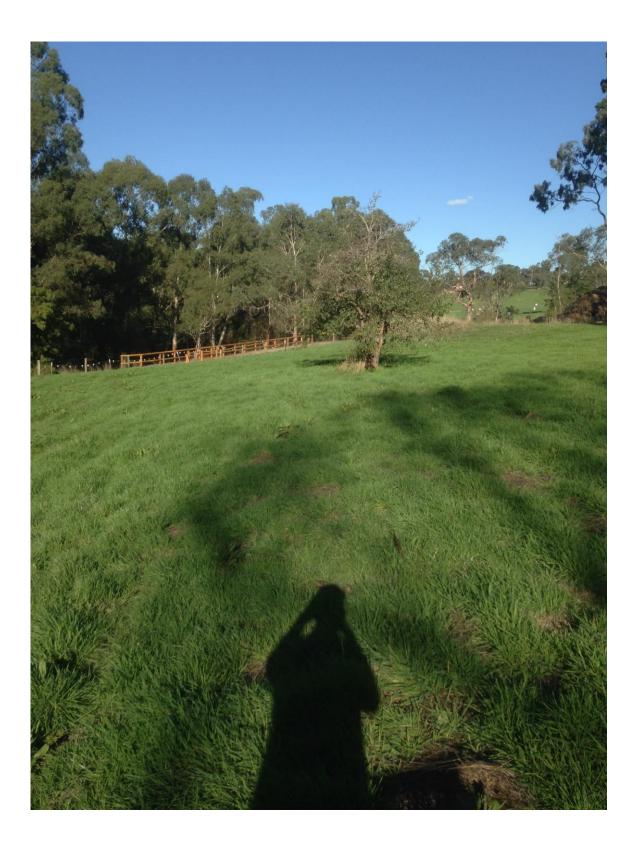














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